Use of Polynomial Shape Function in Shear Deformation Theory for Thick Plate Analysis

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ABSTRACT:-This paper presents use of polynomial shape function in shear deformation theory for thick plate analysis. Total potential energy equation of a thick plate was formulated from the first principle. This equation was subjected to direct variation to obtain three simultaneous direct governing equations for determination of displacement coefficients. Shape (profile) equation for vertical shear stress through the thickness of the plate was formulated from the first principle. From this profile equation, the deformation line equation (called function of z or s) was obtained. This is the model from this study. A numerical problem for a rectangular plate simply supported around all the edges was used to test the sufficiency of this study. Both polynomial and trigonometric shape functions were used in this problem. Three other models were also used. The center deflection - w(0.5, 0.5, 0), in-plane normal x directed stress $\sigma_x(0.5, 0.5, 0.5)$ and x directed vertical shear stress $\tau_{xy}(0, 0.5, 0)$ from the present study for span-depth ratio of 4 of a square plate using polynomial shape function are 0.0055 qa^4/D , 0.03 tqa^2/D and 0.00149 qa^3/D . D is plate flexural rigidity, t is plate thickness, q is the uniformly distributed normal load on plate and a is the primary span of the plate. When Trigonometric shape function is used, the values are 0.0056 qa⁴/D, 0.031 tqa²/D and 0.00147 qa³/D. These are comparable with the value from Pagano (Exact): 0.0052 qa⁴/D, 0.0281 tqa²/D and 0.00117 qa³/D. It is observed that at span-depth ratio of up to 100 the values based on all the models used herein coincide with the Classical plate theory (CPT) values. The values of from CPT based on polynomial shape function are 0.00414 qa⁴/D, 0.0283 tqa²/D and 0.00 qa^3/D .

Keywords: shear deformation, vertical shear stress, stress, deflection, displacement, potential energy shape function

I. INTRODUCTION

Refined plate theories have been characterized by the use of trigonometric displacement function. Many scholars have obtained the closed form solutions and others have obtained approximate solution by use of energy method. However, one thing is common in them all - the use of trigonometric displacement functions to approximate the deformed shapes of the plates. (Chikalthankar et al., 2013; Sayyad, 2011; Akavci, 2007; Sayyad and Ghugal, 2012; Sadrnejad et al., 2009; Daouadji et al., 2013; Hashemi and Arsanjani, 2005; Reddy, 2014; Shimpi and Patel, 2006; Murthy, 1981; Daouadji, Tounsi, Hadji, Henni and El Abbes, 2012; Zhen-qiang, Xiu:xi and Mao-guang, 1994). Others have applied the polynomial displacement functions in numerical methods like finite element method and differential quadrature element methods (Matikainen, Schwab and Mikkola, 2009;Goswami and Becker, 2013, Liu, 2001). In the course of development of refined plate theory, the assumption that the shear deformation line is not varying linear with depth of the plate was introduced. This according to many scholars helps to ensure that the vertical shear stress across the plate section does not remain constant, but varies parabolically with zero values at both the top and bottom surfaces (Kruszewski, 1949; Ambartsumian, 1958 Krishna, 1984; Touratier, 1991; Karama and Mistou, 2003; Sayyad, 2011). They came up with different shear deformation line functions, here-in-after called F(z). However, there F(z) were not strictly based on the vertical shear stress mathematical formulation. If we follow the work of Timoshenko (Timoshenko and Woinowsky-krieger, 1970), we shall note that maximum shear stress occurs at the mid surface (where z = 0) and the value of maximum shear stress is one and half of vertical shear stress. With most of the F(z) from the literature, we may obtain good profile (curve) for the deformation line and shear stress distribution across the section, but the mid surface value of shear stress may not coincide with that from Timoshenko. Thus, the two specific objectives of the present study include

i. To develop a direct governing simultaneous equations for thick plate analysis

ii. To formulation of a polynomial F(z) mathematical in line with works of Timoshenko

iii. To use polynomial displacement functions variational calculus analysis.

II. ASSUMPTIONS

The displacements, u, v and w are small when compared with plate thickness.

i.

- ii. The in-plane displacements, u and v are differentiable in x, y and z axes, while the out-of-plane displacement (deflection), w is only differentiable in x and y axes. This means that the first derivative of w with respect to z is zero. Consequently, $\varepsilon_z = 0$.
- iii. The effect of the out-of-plane normal stress on the gross response of the plate is small when compared with other stresses. Thus, it can be neglected. That is $\sigma_z = 0$.
- iv. The vertical line that is initially normal to the middle surface of the plate before bending is no longer straight nor normal to the middle surface after bending. That is $\phi \neq \theta_c$, where ϕ is the total rotation of the middle surface in this case, θ_c is the classical plate theorem rotation of the middle surface.

III. KINEMATIC RELATIONS

The refined plate theory (RPT) in-plane displacements, u and v as presented on figure 1are defined mathematically as

$$u = u_c + u_s \tag{1}$$
$$v = v_c + v_s \tag{2}$$

Where u and v are in-plane displacements in x and y directions respectively.

The classical in-plane displacements are commonly defined as:

$$u_{c} = -z\theta_{cx} = z\frac{dw}{dx}$$
(3)
$$v_{c} = -z\theta_{cy} = z\frac{dw}{dy}$$
(4)





Analogously, the shear deformation components of the in-plane displacements are defined as:

$$u_{s} = F(z)\theta_{sx}$$
(5)
$$v_{s} = F(z)\theta_{sy}$$
(6)

NoteF(z) is used instead of z. This is because of the fourth assumption (see figure 1). Let us define the deflection (out-of-plane displacement), w as: (7)

 $w = c_1 h$

Substituting equation (7) into equations (3) and (4) gives respectively:

$$u_{c} = -z \frac{d}{dx} (c_{1}h) = -zc_{1} \frac{dh}{dx}$$
(8)
$$v_{c} = -z \frac{d}{dy} (c_{1}h) = -zc_{1} \frac{dh}{dy}$$
(9)

Let us mimic expressions in equations (8) and (9) rewrite equations (5) and (6) as:

$$u_{s} = F(z)c_{2}\frac{dh}{dx}$$
(10)
$$v_{s} = F(z)c_{3}\frac{dh}{dy}$$
(11)

Where c_1 , c_2 and c_3 are coefficients of deflection (w), shear deformation rotations (θ_{sx} and θ_{sy}). Substituting equations (3) and (10) into equation (1) gives:

$$u = (-c_1 z + c_2 F(z)) \frac{dh}{dx}$$
(12)

Similarly, substituting equations (4) and (11) into equation (2) gives:

$$v = (-c_1 z + c_3 F(z)) \frac{dh}{dy}$$
(13)

Strain - Displacement Relations

It was assumed that ε_z is equal to zero. Thus, the remaining five engineering strain components are defined as:

$$\varepsilon_x = \frac{du}{dx} = \left[-c_1 z + c_2 F(z)\right] \frac{d^2 h}{dx^2} \quad (14)$$

$$\varepsilon_y = \frac{dv}{dy} = \left[-c_1 z + c_3 F(z)\right] \frac{d^2 h}{dy^2} \quad (15)$$

$$\gamma_{xy} = \frac{du}{dy} + \frac{dv}{dx} = \left[-c_1 z + c_2 F(z) \right] \frac{d^2 h}{dx dy} + \left[-c_1 z + c_3 F(z) \right] \frac{d^2 h}{dx dy}$$
$$= \left[-2c_1 z + c_2 F(z) + c_3 F(z) \right] \frac{d^2 h}{dx dy} \quad (16)$$

$$\gamma_{xz} = \frac{du}{dz} + \frac{dw}{dx}$$
$$= \left[-c_1 + c_2 \frac{dF(z)}{dz} \right] \frac{dh}{dx} + c_1 \frac{dh}{dx}$$
That is
$$\gamma_{xz} = c_2 \frac{dF(z)}{dz} \frac{dh}{dx}$$
(17)

$$\gamma_{yz} = \frac{dv}{dz} + \frac{dw}{dy}$$
$$= \left[-c_1 + c_3 \frac{dF(z)}{dz} \right] \frac{dh}{dy} + c_1 \frac{dh}{dy}$$
That is
$$\gamma_{yz} = c_3 \frac{dF(z)}{dz} \frac{dh}{dy}$$
(18)

CONSTITUTIVE RELATIONS

It was assumed that σ_z is equal to zero. Thus, the remaining five stress components are defined as:

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IV.

$$\sigma_x = \frac{E}{1 - \mu^2} [\varepsilon_x + \mu \varepsilon_y]$$
(19)

$$\sigma_y = \frac{E}{1 - \mu^2} \left[\mu \varepsilon_x + \varepsilon_y \right]$$
(20)

$$\tau_{xy} = \frac{E(1-\mu)}{1-\mu^2} \gamma_{xy}$$
(21)
$$E(1-\mu)$$

$$\tau_{xz} = \frac{L(1-\mu)}{1-\mu^2} \gamma_{xz}$$
(22)

$$\tau_{yz} = \frac{E(1-\mu)}{1-\mu^2} \gamma_{yz}$$
(23)

V. STRESS – DISPLACEMENT EQUATIONS

Substituting equations (14) to (18) into equations (19) to (23) where appropriate gives:

$$\sigma_x = \frac{E}{1-\mu^2} \left[\left[-c_1 z + c_2 F(z) \right] \frac{d^2 h}{dx^2} + \mu \left[-c_1 z + c_3 F(z) \right] \frac{d^2 h}{dy^2} \right]$$
(24)

$$\sigma_y = \frac{E}{1-\mu^2} \left[\mu z \left(c_1 + \frac{F(z)}{z} B_2 \right) \frac{d^2 h}{dx^2} + \left[-c_1 z + c_3 F(z) \right] \frac{d^2 h}{dy^2} \right]$$
(25)

$$\tau_{xy} = \frac{E(1-\mu)}{2(1-\mu^2)} \left[-2c_1 z + c_2 F(z) + c_3 F(z) \right] \frac{d^2 h}{dx dy}$$
(26)

$$\tau_{xz} = \frac{E(1-\mu)}{2(1-\mu^2)} c_2 \frac{dF(z)}{dz} \frac{dh}{dx}$$
(27)

$$\tau_{yz} = \frac{E(1-\mu)}{2(1-\mu^2)} c_3 \frac{dF(z)}{dz} \frac{dh}{dy}$$
(28)

VI. TOTAL POTENTIAL ENERGY

Total potential energy is the summation of strain energy, \boldsymbol{U} and external work, $\boldsymbol{V}.$ that's

$$\Pi = U + V$$
(29)
Let's define external work as:

$$V = -q \int_{x} \int_{y} w \, dx \, dy$$
(30)
Let's also define strain energy mathematically:

$$c = c \int_{x} \left[c^{\frac{1}{2}} \right]$$

$$U = \int_{x} \int_{y} \left[\int_{-\frac{t}{2}}^{\frac{1}{2}} \sigma \cdot \varepsilon dz \right] dx dy$$

= $\int_{x} \int_{y} \left[\int_{-\frac{t}{2}}^{\frac{t}{2}} \left(\sigma_{x} \varepsilon_{x} + \sigma_{y} \varepsilon_{y} + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right) dz \right] dx dy$ (31)
(4) and (24) (15) and (25) (16) and (26) (17) and (27) and (18) and (28) respe

Using equations (14) and (24), (15) and (25), (16) and (26), (17) and (27), and (18) and (28) respectively gives:

$$\sigma_{x}\varepsilon_{x} = \frac{E}{1-\mu^{2}} \left[\left[z^{2}c_{1}^{2} - 2c_{1}c_{2}zF(z) + c_{2}^{2}F(z)^{2} \right] \left(\frac{d^{2}h}{dx^{2}} \right)^{2} + \mu \left[z^{2}c_{1}^{2} - c_{1}c_{2}zF(z) - c_{1}c_{3}zF(z) + c_{2}c_{3}F(z)^{2} \right] \left(\frac{d^{2}h}{dxdy} \right)^{2} \right]$$
(32)

$$\sigma_{y}\varepsilon_{y} = \frac{E}{1-\mu^{2}} \left[\left[z^{2}c_{1}^{2} - 2c_{1}c_{3}zF(z) + c_{3}^{2}F(z)^{2} \right] \left(\frac{d^{2}h}{dy^{2}} \right)^{2} + \mu \left[z^{2}c_{1}^{2} - c_{1}c_{2}zF(z) - c_{1}c_{3}zF(z) + c_{2}c_{3}F(z)^{2} \right] \left(\frac{d^{2}h}{dxdy} \right)^{2} \right] (33)$$

$$\tau_{xy} \cdot \gamma_{xy} = \frac{E(1-\mu)}{2(1-\mu^{2})} \left[4c_{1}^{2}z^{2} - 4c_{1}c_{2}zF(z) - 4c_{1}c_{3}zF(z) + c_{2}^{2}F(z)^{2} + 2c_{2}c_{3}F(z)^{2} + c_{3}^{2}F(z)^{2} \right] \left(\frac{d^{2}h}{dxdy} \right)^{2} \qquad (34)$$

$$\tau_{xz} \cdot \gamma_{xz} = \frac{E(1-\mu)}{2(1-\mu^{2})}c_{2}^{2} \left[\frac{dF(z)}{dz} \right]^{2} \left(\frac{dh}{dx} \right)^{2} \qquad (35)$$

$$z_{yz} \cdot \gamma_{yz} = \frac{E(1-\mu)}{(1-\mu^{2})}c_{3}^{2} \left[\frac{dF(z)}{dz} \right]^{2} \left(\frac{dh}{dy} \right)^{2} \qquad (36)$$
Substituting equations (32) to (36) into equation (31) gives:

$$U = \frac{D}{2} \int_{x} \int_{y} \left[g_{1}c_{1}^{2} - 2g_{2}c_{1}c_{2} + g_{3}c_{2}^{2} \right] \left(\frac{d^{2}h}{dx^{2}} \right)^{2}$$

$$+ \left[2g_{1}c_{1}^{2} - 2g_{2}c_{1}c_{2} - 2g_{2}c_{1}c_{3} + \frac{1}{2}g_{3}c_{2}^{2} + g_{3}c_{2}c_{3} + \frac{1}{2}g_{3}c_{3}^{2} \right] \left(\frac{d^{2}h}{dxdy} \right)^{2}$$

$$+\mu \left[g_{3}c_{2}c_{3}-\frac{1}{2}g_{3}c_{2}^{2}-\frac{1}{2}g_{3}c_{3}^{2}\right]\left(\frac{d^{2}h}{dxdy}\right)^{2}$$

$$+[g_1c_1^2 - 2g_2c_1c_3 + g_3c_3^2] \left(\frac{d^2h}{dy^2}\right)^2$$

$$+(1-\mu)\frac{\alpha^2}{2}g_4c_2^2\left(\frac{dh}{dx}\right)^2$$

$$+(1-\mu)\frac{\alpha^2}{2}g_4B_3^2\left(\frac{dh}{dy}\right)^2]dxdy$$
 (37)

Where:

$$\overline{D} = \frac{t^3}{12}$$

$$g_1 = \frac{\left(\int_{-\frac{t}{2}}^{\frac{t}{2}} z^2 dz\right)}{\overline{D}} = 1 \quad (38)$$

$$g_{2} = \frac{\left(\int_{-\frac{t}{2}}^{\frac{t}{2}} zF(z)dz\right)}{\overline{D}}$$
(39)
$$g_{3} = \frac{\left(\int_{-\frac{t}{2}}^{\frac{t}{2}} F(z)^{2}dz\right)}{\overline{D}}$$
(40)

$$\alpha^{2}g_{4} = \frac{\left(\int_{-\frac{t}{2}}^{\frac{t}{2}} \left[\frac{dF(z)}{dz}\right]^{2} dz\right)}{\overline{D}} (41)$$

The flexural rigidity of the plate is:
$$D = \frac{E}{1 - \mu^{2}} * \overline{D} = \frac{Et^{3}}{12(1 - \mu^{2})} (42)$$

Let's define the span-depth aspect ratio as
$$\alpha = \frac{a}{t}$$
(43)

Where a and t are the primary span (length in x direction, while b is the length in y direction) of the plate and plate thickness respectively.

Let define non dimensional coordinates R and Q and the span-span aspect ratio, P as:

$$R = \frac{x}{a} \Rightarrow x = aR \qquad (44)$$

$$Q = \frac{y}{b} \Rightarrow y = bQ \qquad (45)$$

$$P = \frac{b}{a} \Rightarrow b = aP \qquad (46)$$
Substituting equations (30), (37) and (43) to (46) into equation (29) gives:
$$\Pi = \frac{abD}{2a^4} \int_0^1 \int_0^1 [g_1c_1^2 - 2g_2c_1c_2 + g_3c_2^2] \left(\frac{d^2h}{dR^2}\right)^2$$

$$+ \frac{1}{P^2} [2g_1c_1^2 - 2g_2c_1c_2 - 2g_2c_1c_3] \left(\frac{d^2h}{dRdQ}\right)^2$$

$$+ \frac{(1 - \mu)}{P^2} g_3c_2c_3 \left(\frac{d^2h}{dRdQ}\right)^2$$

$$+ \frac{(1 - \mu)\alpha^2}{2P^2} [g_3c_2^2 + g_3c_3^2] \left(\frac{d^2h}{dRdQ}\right)^2$$

$$+ \frac{(1 - \mu)\alpha^2}{2P^2} g_4c_3^2 \left(\frac{dh}{dQ}\right)^2] dRdQ$$

$$- ab \int_0^1 \int_0^1 FF dRdQ \qquad (47)$$

VII. DIRECT GOVERNING EQUATIONS

This total potential energy contains three unknown coefficients $(c_1, c_2 \text{ and } c_3)$ for deflection, rotation in x axis and rotation in y axis. Differentiating total potential energy equation with respect to c_1 , c_2 and c_3 in turn will give three simultaneous equations.

$$\frac{d\Pi}{dc_1} = \frac{d\Pi}{dc_2} = \frac{d\Pi}{dc_3} = 0$$
(48)

Substituting equation (47) into equation (48) gives in matrix form:

$$\begin{bmatrix} r_{11} & r_{12} & r_{13} \\ r_{12} & r_{22} & r_{23} \\ r_{13} & r_{23} & r_{33} \end{bmatrix} \begin{bmatrix} c_1 \\ c_2 \\ c_3 \end{bmatrix} = \frac{a^4}{D} \begin{bmatrix} F_{rq} \\ 0 \\ 0 \end{bmatrix} \quad (49)$$
Where
$$r_{11} = g_1(k_1 + 2\frac{k_2}{P^2} + \frac{k_3}{P^4})$$

$$r_{12} = -g_2(k_1 + \frac{k_2}{P^2})$$

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$$r_{13} = -g_2 \left(\frac{k_2}{P^2} + \frac{k_3}{P^4}\right)$$

$$r_{22} = g_3 k_1 + \frac{(1-\mu)}{2P^2} g_3 k_2 + \frac{(1-\mu)\alpha^2}{2} g_4 k_4$$

$$r_{23} = \frac{(1+\mu)}{2P^2} g_3 k_2$$

$$r_{33} = \frac{(1-\mu)}{2P^2} g_3 k_2 + \frac{1}{P^4} g_3 k_3 + \frac{(1-\mu)\alpha^2}{2P^2} g_4 k_5$$

$$k_1 = \int_0^1 \int_0^1 \left(\frac{d^2 h}{dR^2}\right)^2 dR dQ$$

$$k_2 = \int_0^1 \int_0^1 \left(\frac{d^2 h}{dR^2}\right)^2 dR dQ$$

$$k_3 = \int_0^1 \int_0^1 \left(\frac{d^2 h}{dQ^2}\right)^2 dR dQ$$

$$k_4 = \int_0^1 \int_0^1 \left(\frac{dh}{dQ}\right)^2 dR dQ$$

$$k_5 = \int_0^1 \int_0^1 \left(\frac{dh}{dQ}\right)^2 dR dQ$$

$$F_{rq} = \int_0^1 \int_0^1 h dR dQ$$

SHEARING STRESS DISTRIBUTION IN RECTANGULAR CROSS-SECTIONS



Figure 2: A rectangular cross section

From strength of materials, the equation shear stress is given as:

$$\tau = \frac{VH}{lb} \tag{50}$$

Where V, H, I and b are transverse shear force, first moment of area, second moment of inertia and breadth of the section respectively.

Using figure 2 and following mathematically principle the first moment of area is obtained:

$$H = \int z \, dA = \frac{b}{2} \left(\frac{t}{2} - z\right) \left(\frac{t}{2} + z\right)$$

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That is,
$$H = \frac{b}{2} \left(\frac{t^2}{4} - z^2 \right)$$
 (51)

The second moment of inertia for a rectangular section is given as:

$$I = \frac{bt^2}{12} \tag{52}$$

Substituting equations (51) and (52) into equation (50) gives: $\tau = \frac{V}{bt} \left(\frac{3}{2} - 6\frac{z^2}{t^2}\right) = \frac{V}{bt} G(z) \quad (53)$ Where the shear stress profile, G(z) is: $G(z) = \left(\frac{3}{2} - 6\frac{z^2}{t^2}\right) \quad (54)$

It is assumed here that the shear stress profile, G(z) is related to shear deformation profile, F(z) as:

$$G(z) = \frac{dF(z)}{dz}$$
(55)

Using equations (54) and (55) we obtain:

$$F(z) = \frac{3z}{2} \left(1 - \frac{4}{3} \left[\frac{z}{t} \right]^2 \right) (56)$$

$$F(S) = \frac{3St}{2} \left(1 - \frac{4}{3}S^2 \right)$$
(56b)

Where S = z/t (a non-dimensional form of z)

This function of z is exactly Krishna Murty Model (KrishnaMurty, 1984) divided by 1.5. However, using the Krishna Murty model will result in unestimating the vertical shear stress by 50%.

Substituting F(S) models into equations (39) to (41) gives g1, g2, g3 and g4 values for different models used herein for numerical examples:

F(S) =
$$\frac{3St}{2} \left(1 - \frac{4}{3}S^2 \right)$$

 $g_1 = 1; g_2 = 1.2; g_3 = \frac{51}{35}; g_4 = 14.4$

$$F(S) = \frac{l}{\pi} \sin(2\pi S)$$

 $g_1 = 1; \ g_2 = 0.774; \ g_3 = \frac{307}{505}; \ g_4 = 6$

iii. Karama et al. (2003)model

$$F(S) = St. \exp(-2S^2)$$

 $g_1 = 1; g_2 = \frac{344}{439}; g_3 = \frac{274}{422}; g_4 = 6.18744$

VIII.

Recall equation 7: $w = c_1 h$ (7)

Let us rewrite it as:

$$w = c_1 h = B_1 h \left(\frac{q a^4}{D}\right) = k_w \left(\frac{q a^4}{D}\right)$$

This implies that:

$$k_w = B_1 hand B_1 = \frac{c_1}{\left(\frac{qa^4}{D}\right)}$$

Let the displacements of plate under pure bending then be defined as: (ac^4)

$$w = \left(\frac{qa^{4}}{D}\right)k_{w}$$
(56)
$$u = \left(\frac{tqa^{3}}{D}\right)k_{u}$$
(57)
$$v = \left(\frac{tqa^{3}}{D}\right)k_{v}$$
(58)

Where k_w , a and D are as defined earlier, q is the uniform distributed load on the plate. k_u and k_v are extracted from equations (12) and (13) as:

$$k_{u} = (-B_{1}S + B_{2}F(S))\frac{dh}{dR}$$
(59)

$$k_{v} = \frac{1}{P}(-B_{1}S + B_{3}F(S))\frac{dh}{dQ}$$
(60)

$$WhereB_{2} = \frac{c_{2}}{\left(\frac{qa^{4}}{D}\right)}andB_{3} = \frac{c_{3}}{\left(\frac{qa^{4}}{D}\right)}$$

Similarly, let us define the stress components as:

$$\sigma_x = \frac{E}{1 - \mu^2} \left(\frac{tqa^2}{D} \right) k_{\sigma_x}$$
(61)
$$\sigma_y = \frac{E}{1 - \mu^2} \left(\frac{tqa^2}{D} \right) k_{\sigma_y}$$
(62)

$$\tau_{xy} = \frac{E}{1+\mu} \left(\frac{tqa^2}{D} \right) k_{\tau_{xy}}$$
(63)

$$\tau_{xz} = \frac{E}{1+\mu} \left(\frac{qa^3}{D}\right) k_{\tau_{xz}}$$
(64)
$$\tau_{yz} = \frac{E}{1+\mu} \left(\frac{qa^3}{D}\right) k_{\tau_{yz}}$$
(65)

Where $k_{\sigma x}$, $k_{\sigma y}$, $k_{\tau xy}$, $k_{\tau xz}$ and $k_{\tau yz}$ are defined from equations (24) to (28)as:

$$k_{\sigma_{x}} = [-B_{1}S + B_{2}F(S)]\frac{d^{2}h}{dR^{2}} + \frac{\mu}{P^{2}}[-B_{1}S + B_{3}F(S)]\frac{d^{2}h}{dQ^{2}} \quad (66)$$

$$k_{\sigma_{Y}} = \mu[-B_{1}S + B_{2}F(S)]\frac{d^{2}h}{dR^{2}} + \frac{1}{P^{2}}[-B_{1}S + B_{3}F(S)]\frac{d^{2}h}{dQ^{2}} \quad (67)$$

$$k_{\tau_{xy}} = \frac{1}{2P}[-2B_{1}S + B_{2}F(S) + B_{3}F(S)]\frac{d^{2}h}{dRdQ} \quad (68)$$

$$k_{\tau_{xz}} = \frac{B_{2}}{2}\frac{dF(S)}{dS}\frac{dh}{dR} \quad (69)$$

$$k_{\tau_{yz}} = \frac{B_{3}}{2P}\frac{dF(S)}{dS}\frac{dh}{dQ} \quad (70)$$

Substituting equations (42) and (43) into equations (56) to (58) gives: (aa)

$$w = 12(1 - \mu^{2})\alpha^{3}k_{w}\left(\frac{qu}{E}\right)$$
(71)

$$u = 12(1 - \mu^{2})\alpha^{3}k_{u}\left(\frac{tq}{E}\right)$$
(72)

$$v = 12(1 - \mu^{2})\alpha^{3}k_{v}\left(\frac{tq}{E}\right)$$
(73)

Similarly, substituting equations (42) and (43) into equations (61) to (65) gives:

$$\sigma_x = k_{\sigma_x} (12\alpha^2)q$$
(74)

$$\sigma_y = k_{\sigma_y} (12\alpha^2)q$$
(75)

$$\tau_{xy} = k_{\tau_{xy}} [12\alpha^2(1-\mu)]q$$
(76)

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$$\tau_{xz} = k_{\tau_{xz}} [12\alpha^3(1-\mu)]q$$
(77)
$$\tau_{yz} = k_{\tau_{yz}} [12\alpha^3(1-\mu)]q$$
(78)

Let us define non-dimensional form of the displacements and stress components according to Sayyad et al.

$$\overline{w} = \frac{100Ew}{qt\,\alpha^4}(79); \quad \overline{u} = \frac{uE}{qt\,\alpha^3}(80)$$

$$\overline{v} = \frac{vE}{qt\,\alpha^3}(81); \quad \overline{\sigma}_x = \frac{\sigma_x}{q\,\alpha^2}(82)$$

$$\overline{\sigma}_y = \frac{\sigma_y}{q\,\alpha^2}(83); \quad \overline{\tau}_{xy} = \frac{\tau_{xy}}{q\,\alpha^2}(84)$$

$$\overline{\tau}_{xz} = \frac{\tau_{xz}}{q\,\alpha}(85); \quad \overline{\tau}_{yz} = \frac{\tau_{yz}}{q\,\alpha}(86)$$

Using equations (71) to (78), we define the non-dimensional form of the displacements and stress components

$$\bar{w} = 1200(1 - \mu^2)k_w(87)$$

$$\bar{u} = 12(1 - \mu^2)k_u(88)$$

$$\bar{v} = 12(1 - \mu^2)k_v(89)$$

$$\bar{\sigma}_x = 12k_{\sigma_x}(90)$$

$$\bar{\sigma}_y = 12k_{\sigma_y}(91)$$

$$\bar{\tau}_{xy} = 12(1 - \mu)k_{\tau_{xy}}(92)$$

$$\bar{\tau}_{xz} = 12\alpha^2(1 - \mu)k_{\tau_{xz}}(93)$$

$$\bar{\tau}_{yz} = 12\alpha^2(1 - \mu)k_{\tau_{yz}}(94)$$

IX. NUMERICAL PROBLEM

Determine the deflection at the center (0.5, 0.5, 0) of ssss thick plate. Where (0.5, 0.5, 0) means R = 0.5; Q = 0.5; S = 0. Determine also the in-plane normal stresses at (0.5, 0.5, 0.5), in-plane shear stress at (0, 0, 0.5) and the vertical shear stress (τ_{xz}) at (0, 0.5, 0) of thesses plate. Polynomial and trigonometric displacement shall be used. Function.

The polynomial displacement function, h is given as:

 $h = (R - 2R^3 + R^4)(Q - 2Q^3 + Q^4) \quad (57)$

The trigonometric displacement function, h is given as:

 $h = \sin \pi R \sin \pi Q \tag{58}$

The k values for both polynomial and trigonometric functions are given on table 1.

	Polynomial	Trigonometry
k1	0.23621	24.352273
k2	0.23591	24.35227
k3	0.23621	24.35227
k4	0.0239	2.467401
k5	0.0239	2.467401
F _{rq}	0.04	92/227

Table 1: The values of k and Frq for polynomial and trigonometric functions

X. RESULTS AND DISCUSSIONS

A close look at tables 1 to 8 reveals that the values from the polynomial and trigonometric shape functions of the present study are quite close to each other. The recorded difference is attributed to the fact that the two shape functions are only approximations of each other. Two of them are not exactly the same. Furthermore, it is revealed that the values from trigonometric shape function based on the model of present study agree very well with the values based on Krishna model. However, a critical observation of the values reveals that the values from different models are very good approximation of one another. Table 3, 4, 7 and 8 showed that at spandepth ratio of up to 100 and above, the values obtained from the models used herein coincide exactly with values from CPT. This is quite expected since we assumed in CPT analyses that at span-depth ratios of up to 100, a plate can be taking as being thin. Hence, the model derived herein is quite sufficient for thick plate (refined plate theory) analyses. It shall also be deduced that polynomial shape functions are also adequate for thick plate analyses. Most works on thick plate analyses revolve around use of trigonometric shape function and some other functions.

Table 2. Non unnersional parameters for $at = 4$							
a/b = 1	\bar{u}	\overline{W}	$\overline{\sigma_x}$	$\overline{\sigma_y}$	$\overline{\tau_{xy}}$	$\overline{ au_{xz}}$	
Present (P)	-0.0765	6.1044	0.3279	0.3279	-0.1883	0.3906	
Present (T)	-0.0755	6.1383	0.3390	0.3390	-0.1826	0.3854	
Karama (T)	-0.0769	6.1035	0.3452	0.3452	-0.1859	0.3849	
Touratier (T)	-0.0758	6.1342	0.3402	0.3402	-0.1832	0.3973	
Krishna (T)	-0.0756	6.1483	0.3392	0.3392	-0.1826	0.3878	
Pagano (Exact)	0.0720	5.6940	0.3070	-	-	0.3067	
CPT (P)	-0.0723	4.5175	0.3098	0.3098	-0.1779	0.0000	
CPT (T)	-0.0071	4.5434	0.3203	0.3203	-0.1725	0.0000	

Table 2: Nor	dimensional	parameters	for	a/t =	4
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Legend: (P) means the shape function is polynomial (T) means the shape function is trigonometry

Table 3: Non dimensional parameters for a/t = 10

					-	
a/b = 1	\bar{u}	\overline{W}	$\overline{\sigma_x}$	$\overline{\sigma_y}$	$\overline{ au_{xy}}$	$\overline{ au_{xz}}$
Present (P)	-0.0730	4.7723	0.3127	0.3127	-0.1796	0.3920
Present (T)	-0.0720	4.7995	0.3233	0.3233	-0.1741	0.3868
Karama (T)	-0.0723	4.7970	0.3243	0.3243	-0.1746	0.3909
Touratier (T)	-0.0721	4.7991	0.3235	0.3235	-0.1742	0.3991
Krishna (T)	-0.0720	4.8011	0.3233	0.3233	-0.1741	0.3892
Pagano (Exact)	0.0730	4.6390	0.2890	-	-	0.3247
CPT (P)	-0.0723	4.5175	0.30977	0.30977	-0.17792	0.0000
CPT (T)	-0.0714	4.5434	0.3203	0.3203	-0.1725	0.0000

Table 4: Non dimensional parameters for a/t = 100

a/h – 1	$\overline{\eta}$	147	$\overline{\sigma}$	$\overline{\sigma_{}}$	$\overline{\tau_{\dots}}$	$\overline{ au}$
<i>a</i> /0 = 1	u	~~~	υ _χ	- y	• xy	· XZ
Present (P)	-0.0723	4.5201	0.3098	0.3098	-0.1779	0.3920
Present (T)	-0.0714	4.5460	0.3203	0.3203	-0.1725	0.3868
Karama (T)	-0.0714	4.5460	0.3203	0.3203	-0.1725	0.3909
Touratier (T)	-0.0714	4.5460	0.3203	0.3203	-0.1725	0.3991
Krishna (T)	-0.0714	4.5460	0.3203	0.3203	-0.1725	0.3892
CPT (P)	-0.0723	4.5175	0.3098	0.3098	-0.1779	0.3247
CPT (T)	-0.0714	4.5434	0.3203	0.3203	-0.1725	0.0000

Table 5: Non dimensional parameters for a/t = 1000

a/b = 1	\overline{u}	\overline{W}	$\overline{\sigma_x}$	$\overline{\sigma_y}$	$\overline{ au_{xy}}$	$\overline{ au_{xz}}$
Present (P)	-0.0723	4.5175	0.3098	0.3098	-0.1779	0.3923
Present (T)	-0.0714	4.5435	0.3203	0.3203	-0.1725	0.3870
Karama (T)	-0.0714	4.5435	0.3203	0.3203	-0.1725	0.3921
Touratier (T)	-0.0714	4.5435	0.3203	0.3203	-0.1725	0.3994
Krishna (T)	-0.0714	4.5435	0.3203	0.3203	-0.1725	0.3895
CPT (P)	-0.0723	4.5175	0.3098	0.3098	-0.1779	0.0000
CPT (T)	-0.0714	4.5434	0.3203	0.3203	-0.1725	0.0000

	Tuble 6. Dimensional parameters for at = 1							
a/b = 1	u	w	$\sigma_{\rm x}$	$\sigma_{\rm v}$	$ au_{xy}$	$ au_{\mathrm{xz}}$		
	tqa ³ /D	qa ⁴ /D	tqa ² /D	tqa ² /D	tqa ² /D	qa ³ /D		
Present (P)	-0.0070	0.00559	0.03003	0.03003	-0.0172	0.00149		
Present (T)	-0.0069	0.00562	0.03105	0.03105	-0.0167	0.00147		
Karama (T)	-0.0070	0.00559	0.03161	0.03161	-0.0170	0.00147		
Touratier (T)	-0.0069	0.00562	0.03115	0.03115	-0.0168	0.00152		
Krishna (T)	-0.0069	0.00563	0.03106	0.03106	-0.0167	0.00148		
Pagano (Exact)	0.0066	0.00521	0.02811	-	-	0.00117		
CPT (P)	-0.0066	0.00414	0.02837	0.02837	-0.0163	0.00000		
CPT (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000		

Table 6: Dimensional parameters for a/t = 4

Table 7: Dimensional parameters for a/t = 10

a/b = 1	u	W	σ_{x}	σ_{v}	τ_{xy}	$\tau_{\rm xz}$
	tqa ³ /D	qa ⁴ /D	tqa ² /D	tqa²/D	tqa²/D	qa ³ /D
Present (P)	-0.0067	0.00437	0.02863	0.02863	-0.0164	0.00024
Present (T)	-0.0066	0.00440	0.02961	0.02961	-0.0159	0.00024
Karama (T)	-0.0066	0.00439	0.02970	0.02970	-0.0160	0.00024
Touratier (T)	-0.0066	0.00439	0.02962	0.02962	-0.0160	0.00024
Krishna (T)	-0.0066	0.00440	0.02961	0.02961	-0.0159	0.00024
Pagano (Exact)	0.0067	0.00425	0.02647	-	-	0.00020
CPT (P)	-0.0066	0.00414	0.02837	0.02837	-0.0163	0.00000
CPT (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000

Table8:Dimensional parameters for a/t = 100

a/b = 1	u	W	$\sigma_{\rm x}$	$\sigma_{\rm y}$	$\tau_{\rm xy}$	$ au_{\mathrm{xz}}$
	tqa ³ /D	qa ⁴ /D	tqa²/D	tqa ² /D	tqa²/D	qa ³ /D
Present (P)	-0.0066	0.00414	0.02837	0.02837	-0.0163	0.00000
Present (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000
Karama (T)	-0.0065	0.00416	0.02934	0.02934	-0.0158	0.00000
Touratier (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000
Krishna (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000
CPT (P)	-0.0066	0.00414	0.02837	0.02837	-0.0163	0.00000
CPT (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000

Table9:Dimensional parameters for a/t = 1000

a/b = 1	u	W	σ _x	σ_{y}	τ_{xy}	$ au_{\mathrm{xz}}$
	tqa ³ /D	qa ⁴ /D	tqa²/D	tqa²/D	tqa²/D	qa ³ /D
Present (P)	-0.0066	0.00414	0.02837	0.02837	-0.0163	0.00000
Present (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000
Karama (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000
Touratier (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000
Krishna (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000
CPT (P)	-0.0066	0.00414	0.02837	0.02837	-0.0163	0.00000
CPT (T)	-0.0065	0.00416	0.02933	0.02933	-0.0158	0.00000

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